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Evaluating the use of recycled coal combustion products in constructed wetlands: an ecologic-economic modeling approach

Changwoo Ahn, William J. Mitsch *

Environmental Science Graduate Program and School of Natural Resources, The Ohio State University, 2021 Coffey Road, Columbus, OH 43210-1085, USA

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Abstract

A simulation model was developed to couple the biogeochemistry of phosphorus removal with the potentially economical and environmentally beneficial use of a coal combustion waste product as a liner in constructed wetlands. The model includes hydrology, macrophyte and phosphorus submodels coupled to an economic accounting submodel. Data from two constructed wetlands in central Ohio, USA, the Olentangy River Wetland and the Licking County Wetland (LCW), fed by low and high nutrient loads, respectively, were used to calibrate and validate the ecologic portion of the model. The model was used to provide parameters in design of a pilot-scale treatment wetland under construction to test flue-gas-desulfurization (FGD) by-products as a liner material. Subsequent model simulations of the LCW with a liner for prediction of phosphorus retention efficiency showed enhanced phosphorus retention ($\approx 10\%$ by mass) and economic benefits if the wetland were lined with the FGD by-product relative to clay. Total cost saving (liner cost saving plus phosphorus treatment saving) of recycling FGD by-products predicted by the model is closely related to both wetland size and phosphorus loading. Total savings of using FGD by-products as a liner over clay in the LCW (6.4 ha) was calculated at approximately US \$ 23 000 years $^{-1}$ for 30 years at 8% interest rate assumed. © 2002 Elsevier Science B.V. All rights reserved.

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1. Introduction

Nearly 90% of the electricity produced in Ohio, USA, is generated from coal burning and the state

E-mail address: mitsch.1@osu.edu (W.J. Mitsch).

generates about 12% (11.6 million tons) of all coal combustion products (CCPs) in the United States (American Coal Ash Association Survey, 1997). These products, generated in large quantities, have traditionally been treated as wastes and disposed of in expensive, non-productive landfills. However, the disposal of this enormous volume of waste becomes increasingly difficult as landfill

^{*} Corresponding author. Tel.: +1-614-292-9774; fax: +1-614-292-9773.

costs increase and landfill spaces decrease. Flue-gas-desulfurization (FGD) by-product, one of the CCPs, is the result of lime scrubbing of sulfur oxides from flue gases of coal-fired electrical generating stations to reduce acid deposition. More than 7.5 million tons of FGD by-product are produced annually in Ohio, making it the largest single-produced material in the state (Wolfe et al., 2000).

Efforts have been made to reuse FGD wastes in such beneficial ways as in highway and civil engineering applications, as waste-storage pond liners, and as an agricultural liming substitute in livestock feeding pads (Wolfe et al., 2000). One potential use of FGD by-product is as a liner for the construction of treatment wetlands. Ahn et al. (2001) tested FGD by-products as liners in constructed wetlands through 1-m² mesocosm experiments over two years. Their results showed not only the possibility of the material as an alternative liner to commonly used commercial clay or bentonite, but also the potential of additional phosphorus retention in the treatment wetlands as a result of the FGD material itself.

There are three potential benefits of recycling FGD by-products as a liner in constructed wetlands to treat nutrients over natural or commercial clay materials. These benefits are:

- Lower costs for obtaining the recycled FGD by-product for liners as the material is basically free except for handling and hauling costs.
- 2. Reduction in volume of a waste product that is otherwise disposed of in landfills.
- 3. Enhanced phosphorus retention due to the chemical characteristics of the FGD liner material (Ahn et al., 2001).

Dynamic models of phosphorus retention in wetlands have been studied extensively (Kadlec and Hammer, 1988; Mitsch and Reeder, 1991; Kadlec, 1997; Richardson et al., 1997; Wang and Mitsch, 2000). Some studies have also attempted to interlink ecology model and economic analysis (Baker et al., 1991; Breaux et al., 1995; Grant and Thompson, 1997; Robles-Diaz-de-Leon and Nava-Tudela, 1998; Cardoch et al., 2000). Few studies, however, have been conducted to connect

ecological functions of constructed wetlands (e.g. phosphorus retention) with their economic consequences through a combined ecologic-economic modeling approach.

The goal of our study was to develop a dynamic model which simulates phosphorus retention incorporated with economic benefits of recycling FGD waste as a liner in constructed wetlands. This model allows an a priori cost saving calculation of recycling FGD by-product as liners relative to using clay material.

2. Site description

2.1. Olentangy River wetland (ORW)

Two 1-ha experimental wetland basins of the Olentangy River Wetland Research Park (ORWRP) in Columbus, Ohio were constructed on alluvial, old-field soils adjacent to the third-order Olentangy River in 1994 (Mitsch et al., 1998). The wetlands are fed by the Olentangy River water. Nairn and Mitsch (2000) and Spieles and Mitsch (2000) described phosphorus and nitrogen retention in these experimental basins in detail.

2.2. A pilot-scale FGD-lined wetland system at the ORWRP

A medium-scale FGD-lined wetland study is currently underway at the ORWRP. This is a larger-scale effort than the mesocosm studies (Ahn et al., 2001) to further investigate the effects of FGD by-product recycled as liners in treatment wetlands. We expect this pilot-scale wetland study, being conducted over the next two years (2001–2002), to provide essential information before going to full-scale application of FGD by-products in building wetlands.

Four separate pilot wetland basins were constructed ($\approx 3~\text{m} \times 7.8~\text{m} \times 1.5~\text{m}$). All basins were placed in parallel. A 0.15-cm plastic liner and a geo-membrane such as those used in landfill caps were fitted to the four basins and welded appropriately so that the material cov-

ered both the wetland basins and the berms in between the basins. A layer of gravel approximately 0.2-0.3 m deep was then added to the cells to serve as the subsurface strata of these basins. FGD by-product was then applied to two of the basins and the berms in between. and compacted by excavating machinery to 0.3 m. Recompacted clay was applied to the other two basins in the same fashion. Approximately 0.3-m site soil obtained during the excavation was then added to all four basins as a medium for wetland vegetation to grow in. We used the dynamic model we developed in this study to suggest design parameters for this pilot-scale wetland. Hydraulic loading rate (cm days⁻¹) and inflow TP concentration were manipulated in the model to achieve optimal conditions for phosphorus retention in the model simulations.

2.3. Licking County Wetland

Licking County Wetland (LCW), located near Kirkersville, Ohio, was constructed in 1995 for the tertiary treatment of municipal wastewater effluent from the Southwest Licking Community Water and Sewer District treatment plant (Mitsch and Metzker, 1996; Spieles and Mitsch, 2000). The wetland site consists of two 3.2-ha basins built on alluvial, previously farmed soils which discharge water into the South Fork of the Licking River, a second order stream. Secondarily treated wastewater has fed both the Wetland North (LCWN) and the Wetland South (LCWS) since the spring of 1995. Wetland South (LCWS), however, proved to be leaky and did not retain water in 1996. Subsequently, all wastewater was routed to LCWN for the duration of the study. This specific condition of LCWS may provide a good case for testing FGD waste as a liner in the future as our pilotscale wetland study on the material reveals more information. The model was thus applied to the LCW to simulate its ecologic-economic dynamics when lined with either clay or FGD byproduct. The simulations calculated the potential cost savings of recycling FGD by-products as a liner as well as phosphorus retention efficiency in the LCW.

3. Simulation methods

The goal of the ecologic-economic wetland model was to enable prediction of phosphorus retention and estimation of cost saving of building wetlands lined with recycled FGD waste. Hydraulic loading rates and total phosphorus (TP) inflow concentrations over a 2-year period (1996-1997) from ORW basin 1 (ORW 1) and LCWN were used as input for model calibration and validation to investigate general performance of the model in phosphorus retention. All initial conditions for the model were obtained from ORW 1 through previous studies (e.g. Harter and Mitsch, 1999). Four submodels were developed such as hydrology, macrophyte, phosphorus and economic submodels. Each submodel was linked to the previous submodel(s) and calibrated. A set of nonlinear, ordinary differential equations was used to describe the submodels. The model was integrated using the software STELLATM V, a high level visual-oriented programming and simulation language for use on Apple MacintoshTM computers (Richand Peterson. 1997). Fourth-order mond Runge-Kutta was used as the integration method with a time step of 0.1 weeks. Simulations were designed to run over a 2-years period (from 1 January of year 1 to 31 December of year 2) in constructed surface-flow wetlands. Calibration was carried out by adjusting selected parameters in the model to obtain a best-fit between model estimations and field data from ORW 1 for TP retention. A stepwise method (Mitsch and Reeder, 1991; Wang and Mitsch, 2000) was used in this process by first calibrating the hydrology submodel, then macrophyte submodel, phosphorus submodel, and finally economic model. At each step, values of parameters determined during the previous step were not allowed to change from previously calibrated values.

Sensitivity analysis is usually performed during model simulations to find the most important parameters which determines the main state variables of interest (Jørgensen, 1988). Wang and Mitsch (2000) verified that TP inflow concentration and phosphorus sedimentation coeffi-

cient (sed k in our model) were the most sensitive parameters explaining TP retention in a dynamic phosphorus model of created wetlands. Therefore, we carried out sensitivity analysis of TP inflow concentration and the sedimentation coefficient (sed k) on corresponding changes of TP retention (%) in our model runs with the LCW. The selected parameters were varied by +2 orders of magnitude for TP inflow concentration and by +10 through 80% for sed k. Sensitivity analysis was also conducted in the economic submodel to investigate the effects of changing inflow phosphorus loading on potential economic benefits of FGD-lined wetlands. Total annualized cost saving estimates were based on a 30-year lifetime and 8% interest rate following current industry practices. The sensitivity of these assumptions was also investigated. Assumptions were made in developing the ecologiceconomic wetland model, including the following:

- 1. Vegetation uptake of phosphorus is from sediments and not from the water column (Richardson, 1985);
- 2. Phosphorus sedimentation is influenced by plant biomass (Kadlec and Knight, 1996);
- 3. A slight toxic effect of FGD by-product on early development of macrophyte is expected (Ahn and Mitsch, 2001);
- 4. Enhanced phosphorus retention occurs because of the FGD material (Ahn et al., 2001) and the rate is similar over the first 2-year period of wetland operation;
- The growing season for the model is for midtemperate regions and begins early April and ends mid September;
- 6. Seepage from the wetland basin is assumed zero with any liner being applied; and
- No matter which material, either FGD byproduct or clay, is used as a liner it needs to be hauled to the wetland site from a similar distance.

4. Model description and calibration

A conceptual model of the constructed wetland with economic system is shown in Fig. 1. Differential equations used for the model are presented in Table 1 and state variables, forcing functions and

parameters are summarized in Table 2. The model is described in detail below.

4.1. Hydrology submodel

The hydrology submodel has only one state variable, water volume (V), which balances a pumped inflow, seepage and a surface outflow from the wetland. The hydraulic inflow loading based on field data collected over the 2-year period (1996–1997) at ORW 1 (Table 3) was used in model calibration. Surface outflows were predicted by regression with wetland volume data for both ORW 1 and LCWN. Seepage to groundwater was included as a function of wetland area. Seepage coefficient (SC) was calibrated based on field estimation of seepage. In simulations of a constructed wetland with any liner being applied, either clay or FGD by-product, seepage was set to zero in the model.

4.2. Macrophyte submodel

The macrophyte submodel includes two state variables: biomass and detritus. Macrophyte production included only aboveground biomass in this model. A solar efficiency of 2.5% (e.g. Wang and Mitsch, 2000) was estimated and applied to simulate net primary productivity of macrophytes in the submodel. An estimated 20% reduction in plant growth was applied as a toxicity effect with a FGD material (Ahn and Mitsch, 2001) only during the first growing season (13–38th week) of the wetland model simulated with a FGD liner. An 0.8% recovery per week was applied to simulate the mitigation of toxicity effects over that time as the potential toxicity in experiments by Ahn and Mitsch (2001) was observed to lessen, and was negligible in the second year with full-grown plants. The toxicity factor was designed as a conditional sentence with an on/off switch (Table 1). It was also assumed that the dominant vegetation in this system is Schoenoplectus tabernaemontani, the most common species in the constructed wetlands explored in this study ($\approx 90\%$ of vegetation cover).

Net primary productivity (NPP) depends on solar energy, length of growing season, and FGD toxicity factor. Frost is a pulse function that occurs

on the 41st week of the year. It signifies the first frost of the season that dispatches the living biomass stand into detritus. One other variable, standing stock, is included in the macrophyte submodel, and is connected to the phosphorus submodel. Standing stock, the sum of biomass and

detritus held above the surface of the wetland substrate, is assumed to influence water movement and so positively affects the phosphorus sedimentation rate in the phosphorus submodel whether alive or dead. A similar approach in modeling macrophyte dynamics was used in Baker et al. (1991) and

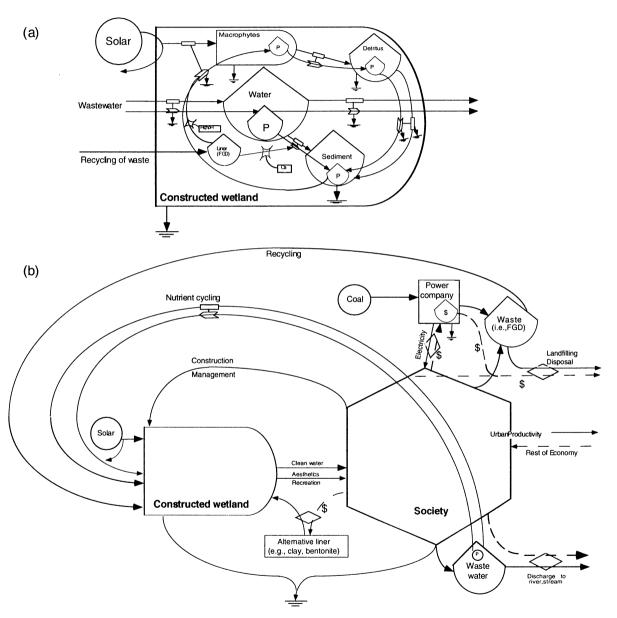


Fig. 1. Conceptual model of ecological-economic system of a constructed wetland with recycled coal combustion wastes: (a) detail phosphorus processes in a constructed wetland including FGD liner; and (b) connection between a constructed wetland shown in (a) with economics and society.

Table 1
Differential equations used in the ecologic-economic wetland model

Hydrology submodel

dV/dt(t) = Inflow - Outflow - Seepage

Where

V Wetland water volume (m³)

Inflow Pumped inflow from the field (m³ week⁻¹)

Outflow Regression, surface outflow (m³ week⁻¹) ORW: $(3.8 \times 10^{-5} \times V^2) + (2.8 \times V)$

LCWN: $(2.1 \times 10^{-4} \times V^2) + (0.6 \times V)$

Seepage If (FGD factor = 1) then (0) else (V/Depth SC), seepage to groundwater

 $(m^3 \text{ week}^{-1})$

Depth V/Aw, average water depth (m)

Aw Wetland area (m²)

SC Seepage coefficient (m week⁻¹)

Tr [V/((Inflow + Outflow)/2)]/7, hydraulic retention time (days)

Macrophyte submodel

dBiomass/dt = NPP flow-Loss dDetritus/dt = Loss-Decay

Where

Biomass Macrophyte biomass in the wetland (g)

NPP flow Solar \times MAC se \times GS \times FGDtf/R \times Aw, macrophyte productivity (g week⁻¹)

Loss Biomass \times (0.0007 + frost), amount of energy entering the detritus from biomass

(g week⁻¹)

Detritus (g)

Decay Detritus \times Decay R \times FT, amount of energy lost from the detritus (g week⁻¹)

Solar $4000-2000 \times \cos(2 \times \pi \times (\text{time})/52)$, amount of solar energy flowing into the

wetland (kcal m⁻² week⁻¹) Macrophytes solar efficiency

MAC se Macrophytes solar efficiency GS Growing season for biomass

FGDtf If (FGD factor = 1) then (Tx) else (1), FGD toxicity factor Tx Temporal pattern of initial toxicity of FGD being applied

R Energy per biomass ratio (kcal g^{-1})

Frost Pulse function that occurs on the week 41 out of 52 weeks

Decay R Detritus decay rate (week $^{-1}$)

FT 1.06 $^{\circ}$ (Wtemp-20), temperature function for decay Wtemp 15–13 \times cos(2 \times π \times (time)/52), water temperature (°C)

Standing stock Biomass + Detritus (g)

Phosphorus submodel

dBiomass P/dt = Uptake -loss P

Where

Biomass P Amount of phosphorus found in biomass (g)

Uptake NPP flow × UTe, amount of phosphorus entering the biomass from sediments

 $(g \text{ week}^{-1})$

Loss P Loss × Le, amount of phosphorus entering the detritus from biomass (g week⁻¹)

UTe Uptake efficiency Le Loss efficiency

dDetritus P/dt = Loss P - decomp P

Where

Detritus P Amount of phosphorus found in detritus (g)

Decomp P Decay × De, amount of phosphorus being added to the sediment from detritus

through decomposition (g week⁻¹)

De Decay efficiency

Table 1 (Continued)

dSediment P/dt = Decompo P + ST-Uptake

Where

Sediment P

ST

Amount of phosphorus found in the sediment (g)

Amount of phosphorus found in the water (g)

If (standing stock $< 4 \times 10^6$) then (Water P \times STC/depth) else ((standing

Inflow × Inconc, amount of phosphorus entering the water column

 $stock \times 5 \times 10^{-8} + STC$)/depth × Water P), amount of phosphorus entering

the sediment from water (g week⁻¹)

Sed kPhosphorus sedimentation velocity (m week⁻¹)

dWater P/dt = Inload - Outload - ST-P seepage-FGD

effect

Where

Water P

Inload

 $(g \text{ week}^{-1})$ Water $P \times Outflow/V(g week^{-1})$ Outload

P seepage

If (FGD factor = 1) then (0) else (seepage \times (Inconc + Outconc)/2), phosphorus loss from water column through seepage (g week⁻¹)

FGD effect If (FGD factor = 1) then (Water $P \times FGD$ CaP) else (0), additional phosphorus retention by FGD (g week⁻¹)

Total phosphorus concentration of inflow from the field $(g m^{-3})$ Inconc Outconc Water P/V, total phosphorus concentration of outflow (g m $^{-3}$)

FGD CaP Ca-P precipitation efficiency

P removal conc (In conc – out conc)/In conc × 100, percent removal of phosphorus based

on concentration (%)

P removal load (Inload – outload)/Inload × 100, Percent removal of phosphorus based on

load (%)

P removed with clay (Inload – outload), amount of phosphorus removed with clay liner

(g week⁻¹)

P removed with FGD (Inload – outload) × 1.1, amount of phosphorus with FGD liner

(g week⁻¹), 10% additional P retention assumed (e.g. Ahn et al., 2001)

Economic accounting submodel

Liner cost saving - PMT (Interest rate, NY, Liner saving, 0), annualized payment on the

> capital cost of a constructed wetland with FGD wastes (\$ year⁻¹); PMT function returns a negative value, indicating that the payment is an expense, so (-) is applied to the PMT to produce (+) value of the

saving from the recycling of FGD wastes as liners.

Cost for wetland $196336 \times (Wt area)^{(-0.511)} \times (Wt area)$, regression equation for

calculating the cost of wetland construction from Mitsch and Gosselink

(2000) (\$)

P treatment saving (P removed with FGD-P removed with clay) × Unit cost of P

wetland \times 52 \times 2, amount of money saved by the enhanced removal of

phosphorus due to FGD by-products (\$ year⁻¹)

Interest rate 0.08, annual interest assumed (8%)

NY 30, lifetime of constructed wetland assumed (year) Annualized cost saving + FGD treat saving (\$ year⁻¹) Total savings

Liner cost Cost for wetland construction \times 0.2, approximately 20% of the

construction cost is for liner (\$)

Unit cost of P wetland $0.1781 \times \text{In conc}^{\, (-0.7151)}$, regression equation developed on unit cost

of phosphorus removal in constructed wetlands (\$ g-P^-1)

Total area of wetland constructed (6.4 for LCW) (ha) Wt area

Table 2 State variables, forcing functions and parameters for the ecologic-economic wetland model

Symbol	Name	Value/units	Source
State variables			
V	Water volume of the wetland	m^3	Calculation, ORW
Biomass	Macrophyte biomass in the wetland	g	Harter and Mitsch (1999)
Detritus	Detritus	g	Harter and Mitsch (1999)
Biomass P	Amount of phosphorus found in biomass	g	Calculation, ORW
Detritus P	Amount of phosphorus found in detritus	g	Calculation, ORW (based on Jørgensen et al., 1991)
Sediment P	Amount of phosphorus found in the sediment	g	Calculation, ORW
Water P	Amount of phosphorus found in the water	g	Calculation, ORW
Forcing functions			
Inflow	Pumped inflow	m ³ week ⁻¹	
FGD liner	FGD waste being used as a liner in constructed wetland		
Solar	Amount of solar energy flowing into the wetland	kcal m ⁻² week ⁻¹	
Inconc	Total phosphorus concentration of inflow	${\rm g}{\rm m}^{-3}$	
Parameters and c	oefficients		
Aw	Area of the wetland	m^2	Field
Depth	Average water depth	m	Calculation
SC	Seepage coefficient	0.1 m week ⁻¹	Calibration
Decay R	Detritus decay rate	0.035 week^{-1}	Mitsch and Gosselink (2000)
Frost	Pulse function	1.5 pulse	Calibration
MAC se	Macrophytes solar efficiency	0.025	Estimate
GS	Graph of growing season for biomass	0.0-1.0 range	Odum (1971)
R	Energy per biomass ratio	4.1 kcal g^{-1}	Boyd (1970)
UTe	Uptake efficiency	0.0028	Calibration
Le	Loss efficiency	0.05 week ⁻¹	Calibration
De	Decay efficiency	0.0038	Calibration
Standing stock	Biomass + Detritus	g	Calculation
sed k	Phosphorus sedimentation velocity	0.1 m week ⁻¹	Calibration
FGD CaP	Ca–P precipitation efficiency	0.82	Calibration (see Ahn et al., 2001)

Flanagan et al. (1994). The standing stock serves as a set of 'living weirs' that reduce the velocity of the inflow, thereby enhancing physical sedimentation of phosphorus. Physical sedimentation is the major pathway of phosphorus retention through wetlands (Wang and Mitsch, 2000). The effect of temperature on detritus decay was assumed to follow an exponential function based on Brown and Barnwell (1987).

4.3. Phosphorus submodel

The phosphorus submodel consists of four phosphorus pools and numerous auxiliary variables and pathways, describing general phosphorus dynamics

in constructed wetlands (Table 1). Biomass P, Detritus P, Sediment P, and Water P are the four main state variables in this submodel (Table 2). Water P, the amount of phosphorus in water, has one inflow (Inload) and four outflows (Outload, Sedimentation, P seepage and FGD effect) (Table 1). Inload is dependent on pumped water inflow (m³ week -1) and phosphorus concentration (g m -3). Outload carries phosphorus out of the system and is also dependent upon amount of phosphorus left in water column, water outflow, and the volume of water in the wetland (Table 1). Sedimentation was the second outflow, thus removing phosphorus into the sediments. Sedimentation is controlled by several factors including amount of

phosphorus in water column and the amount of standing stock in the wetland. Sedimentation coefficient (sed k) was obtained through model calibration to find a better-fit between observed and simulated percent TP removal. Phosphorus seepage accounted for a certain amount of phosphorus lost from the water column through seepage, but this term is designed to become zero when liners are used in the wetlands due to the assumption of no seepage. FGD effect was included in the model as the fourth outflow of Water P. To simulate the enhanced P removal observed through the experiments by Ahn et al. (2001), approximately 10% more phosphorus (as mass) was simulated to be removed from the water column in wetlands being built with FGD liners. A potentially negative impact of FGD materials on phosphorus retention, such as decreasing sedimentation by lowered standing stock when FGD toxicity is active, is also included in the model. Most phosphorus used by macrophytes is taken up from sediments (Wang and Mitsch, 2000) and is assumed to be proportional to net primary productivity (NPP flow) of the wetland macrophyte. Loss of phosphorus in biomass to detritus, and then to the sediment through decomposition processes was generally simulated as a linear pathway. Calibration efforts were generally focused on percent phosphorus removal prediction in constructed wetlands.

Table 3 Hydrology, phosphorus loading data (mean \pm S.E., (n)) for the Olentangy River Wetland (ORW) and the Licking County Wetland (LCW), 1996–1997

Parameters	ORW basin 1	LCW north basin
Area (ha)	1	3.2
Inflow $(m^3 day^{-1})$	$932 \pm 40 (75)$	$3.138 \pm 127 (87)$
Hydraulic loading rate (cm day ⁻¹)	$9.3 \pm 0.4 (75)$	$9.8 \pm 0.40 \ (87)$
Retention time (day)	$2.1 \pm 0.3 (75)$	$2.6 \pm 0.2 \ (87)$
Inflow P concentration (mg P l ⁻¹)	0.16 ± 0.01 (75)	1.19 ± 0.15 (87)
P removal by concentration (%)	$38.9 \pm 1.1 \ (75)$	$20 \pm 1.2 \ (87)$

4.4. Economic accounting submodel

Two different aspects of potential cost saving from recycling FGD wastes as liners in constructed wetlands were explored in this submodel. One was a cost saving from wetland construction with FGD liners relative to commonly used clay materials, which provide useful a priori information to managers and decision-makers on treatment wetlands. The cost of wetland construction varies widely. depending on the location, type, size, and objectives of the wetland (Mitsch and Gosselink, 2000). A strong relationship between wetland cost per area and wetland size found by Mitsch and Gosselink (2000) based on various cases of treatment wetlands in the US, including the ORW and the LCW, was used in our economic accounting submodel to calculate cost of wetland construction.

$$C_{\rm A} = \$196\ 336 \times {\rm A}^{-0.511}$$
 $n = 15$ $R^2 = 0.785$ (1)

where C_A is the capital cost of wetland construction per unit area (\$ ha⁻¹), and A is the wetland area (ha).

Cost of liner material is generally reported to comprise 20–25% of total wetland construction cost (Kadlec et al., 2000). In the model, we applied 20% of the total estimated cost of wetland construction conservatively as the potential saving of using FGD waste relative to clay being purchased. The capital saving of FGD liner was converted into an annualized saving based on an assumed 8% interest rate and 30-year lifetime of a constructed wetland.

The other possible saving from using FGD waste as a liner results from cost of phosphorus removal in constructed wetlands. Byström (1998) estimated nitrogen removal cost in wetlands by linking a function for construction costs of wetlands with a function that defines the nitrogen removal capacity of wetlands. Similarly, in our study, a possible cost saving (\$ year - 1) was calculated by multiplying the unit cost of removing 1 g of TP from surface inflow by the amount of phosphorus being additionally removed due to FGD by-product compared to clay (see Table 1).

The unit cost of phosphorus removal would be a useful tool in the development plans for a reliable,

Table 4 Simulations run of the ecologic-economic wetland model in this study

Simulation	Number of simulations
Calibrating model of ecosystem (phospho	rus)
(1) Calibration of model with the	1
ORW ^a data in 1996–1997 (2) Validation of model with the LCW ^b data in 1996–1997	1
Sensitivity analysis (with LCW)	
(3) Phosphorus sedimentation coefficient (sed <i>k</i>)	t 5
(4) Interest rate and wetland life expectancy	4
(5) Total phosphorus inflow concentration	5
Simulations to design a pilot-scale wetlan (6) Manipulating hydraulic loading rate and inflow TP concentration (7 different P loading rate simulated)	nd lined with FGD 7
Real-world application of the model with (7) LCW with clay liner (no seepage) (8) LCW with FGD liner (no seepage+FGD effects)	LCW 1

^a Olentangy River Wetland; data from ORW basin 1 were used.

implementable treatment system at a reasonable cost. Very few studies have been conducted on how much it would cost to treat a unit mass of phosphorus through wetland treatment technology. In order to quantify the removal cost of a unit mass of phosphorus in our study, we used data available from real-world examples including North American Wetland Database (NAWDB, 1993), ORW and LCW to obtain construction costs of those wetlands. Those capital construction costs were converted into periodic series of annualized payments for a 30 years period at an 8% interest rate. and then added it to annual operation and maintenance costs of those wetlands to calculate the total annual costs. The calculated total annual costs were adjusted to January 2000 by multiplying by 1.176 based on Mean history cost index (Means Company Inc., 1999) as most construction costs adopted in the calculations were in 1993 dollars. The total

annualized cost for each constructed wetland calculated was then divided by total annual amount of phosphorus removed through it, thus resulting in unit cost (\$) of 1 g of phosphorus removed. Values of all other possible services being provided by constructed wetlands such as recreation, biodiversity and removal of other nutrients or pollutants. although important, are not included in this unit cost estimation. Amount of phosphorus entering the water column (Inload, kg-P ha⁻¹ day⁻¹) and TP concentration of inflow (Inconc, $g m^{-3}$) were found to be closely related to unit cost of phosphorus removed in constructed wetlands in a previous study (Brown and Caldwell Consultants, 1993). Therefore, we established two power functions to describe the relationship between unit cost and phosphorus loading in constructed wetlands:

Unit cost = $0.0673 \times Inload^{(-0.8189)}$

$$n = 5 R^2 = 0.5375 (2)$$

Unit cost = $0.1781 \times Inconc^{(-0.7151)}$

$$n = 5 R^2 = 0.9753 (3)$$

where Unit cost is the cost of removing 1 g of TP (US \$ g-P⁻¹), Inload is the amount of phosphorus entering the water column (kg-P ha⁻¹ day⁻¹), and Inconc is the TP concentration of inflow (g m⁻³).

In the above equations, the unit cost of phosphorus removed decreases drastically as TP inflow concentration increases as found in Brown and Caldwell Consultants (1993). Inflow TP concentration explained almost 98% of the variance of unit cost of removing 1 g of TP through constructed wetlands. Therefore, the second equation was adopted in our model to link the phosphorus submodel to the economic accounting submodel.

5. Results and discussion

The model was used to simulate ecologic-economic dynamics of phosphorus retention in constructed wetlands. Table 4 summarizes simulations performed in this study.

5.1. Simulation results-calibration and validation

The process of calibration consists of adjusting

^b Licking County Wetland; data from the north basin (LCWN) were used.

key model parameters so that simulated values for a modeled variable (TP outflow concentration or % TP removal) are in agreement with observed field data. The hydrology submodel was the first submodel to be calibrated, and showed less than 10% difference for average surface outflow at each time step (m³ week -1) between field data and model outputs. Water depth was predicted almost the same as their field measurements for two different sites (ORW 1:0.2 m; LCWN: 0.25 m), resulting in less than 5% difference.

Model performance in predicting phosphorus retention is presented in Fig. 2. Percent phosphorus removal is used as a criterion for evaluating the model's performance. The calibrated model predicted 0.092 g-P m⁻³ for TP outflow concentration, thus achieving 43.2% TP removal on average over a 2-year period through the ORW 1. Cali-

brated% TP removal was in good agreement with the actual percent TP removal of the ORW 1 (38.9% on average), showing about 10% error from the actual retention. The LCWN used for validation showed a 20% difference between field-measured and simulated phosphorus retention but this margin of error is quite acceptable in this type of general prediction over a 2-year period.

Fig. 3 shows phosphorus dynamics simulated for the four main state variables such as biomass, detritus, sediment and water column in the LCWN. This simulation describes plant—soil—water interactions fairly well in the wetland ecosystem. Phosphorus in the water (Water P) fluctuated between 0 and 1.27 g-P m⁻² with an average value of 0.22 g-P m⁻² over a 2-year period. Water P remained low during the growing season and showed relatively higher peaks as the growing season ends.

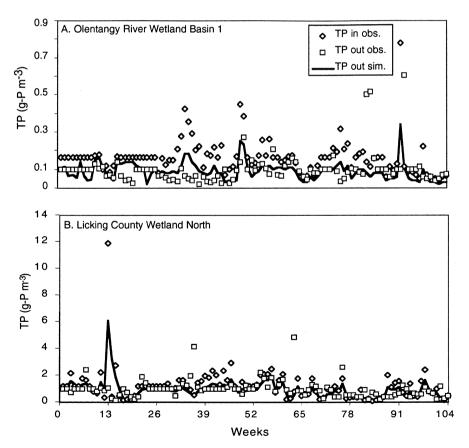


Fig. 2. Calibration and validation of phosphorus retention model showing simulated and actual total phosphorus concentration of outflow for (a) ORW 1 and (b) LCWN in Ohio, USA over a 2-year period (1996–1997).

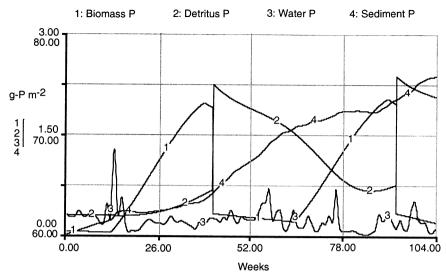


Fig. 3. Simulated phosphorus dynamics in biomass, detritus, water and sediment in LCW over a 2-year period (1996-1997) in this study.

Biomass production and standing stock start as the growing season begins, which both influence sedimentation of phosphorus positively. Therefore, phosphorus in the water column is closely affected by the macrophyte submodel, and hence shows a kind of seasonality. Phosphorus in plant biomass fluctuates between about 0.03 and 2.03 g-P m⁻² in the LCW simulation seasonally because of its dependence on the macrophyte submodel. As NPP flow increases, uptake of phosphorus from the sediment also increases (Fig. 3). The outflow pathway of biomass phosphorus is directly dependent on the loss rate of biomass to detritus in the macrophyte submodel (Table 1). Biomass P is also affected greatly by the occurrence of frost which terminates the growing season on week 41 in the first year and on week 93 in the second year. The phosphorus in detritus (Detritus P) follows an inverse pattern of the phosphorus in biomass. As biomass P decreases on around week 41, the phosphorus flows from biomass to detritus. Phosphorus in the sediment (Sediment P) increases over time and reaches a peak of 76 g-P m⁻² over a 2-year period in the LCW, but it shows relatively lower rate of increase during the growing seasons non-growing compared to seasons macrophyte uptake of phosphorus from sediments drastically increases (Fig. 3).

5.2. Sensitivity analysis

Fig. 4 shows how percent TP removal of wetlands changes as phosphorus sedimentation coefficient (sed k) changes. TP retention in wetlands increases as sed k increases, thus indicating that phosphorus sedimentation is a significant process contributing to phosphorus retention efficiency of our wetland model. Inflow TP concentration was also varied from the inflow TP concentration of the LCWN by ± 2 orders of magnitude, which did not significantly influence percent TP removal in the wetland model when the inflow TP concentration was higher than 0.12 g m⁻³ as shown in Fig. 5.

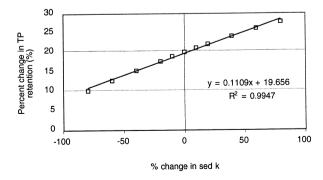


Fig. 4. Sensitivity of percent change in total phosphorus retention to phosphorus sedimentation coefficient (sed *k*).

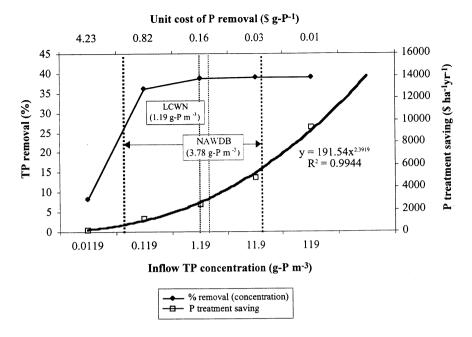


Fig. 5. Relationship of TP removal (%), unit cost of P removal, and P treatment savings along a gradient of inflow TP concentration in the calibrated model simulating LCW in this study. Two thick, vertically dotted lines indicate minimum and maximum values of inflow TP from North American Treatment Wetland Database (NAWDB, 1993). LCWN is Licking County Wetland North basin. Values in the boxes are mean inflow TP concentrations for LCWN and NAWDB.

Inflow TP concentration of $0.12~{\rm g~m^{-3}}$ is usually regarded a low-P condition as in the river inflow of the ORW (Spieles and Mitsch, 2000). Most treatment wetlands are found to have higher inflow TP concentrations than $0.12~{\rm g~m^{-3}}$ (Kadlec and Knight, 1996). Therefore, TP retention predicted in our model may not be sensitive to change in the inflow TP concentrations of treatment wetlands. Rapidly decreasing percent TP removal was observed when Inflow TP concentration was below $0.2~{\rm g~m^{-3}}$ (Fig. 5).

5.3. Simulations for the pilot-scale wetland with a FGD liner

We ran the dynamic model with a variety of combinations of hydraulic loading rates and inflow TP concentrations for the pilot-scale wetland (≈ 3 m $\times 7.8$ m $\times 1.5$ m) lined with FGD by-product currently under construction at the ORWRP. Table 5 shows those simulation results. Mitsch and Gosselink (2000) reported that loading rates to surface flow wetlands for wastewater treatment from small

municipalities ranged from 1.4 to 22 cm day⁻¹ $(average = 5.4 \text{ cm day}^{-1})$. Knight (1990) recommended a rate of 2.5–5 cm day ⁻¹ for surface water systems. The rate of 5-10 cm day⁻¹ was also maintained for the ORW and LCW since they were constructed (Spieles and Mitsch, 2000). Therefore, we chose 5 cm day⁻¹ as target inflow loading rate, one of the design parameters for this pilot-scale wetland, and ran the model while varying the value within a reasonable range from 2.5 to 15 cm day $^{-1}$. Inflow TP concentration was also varied from 0.1 to 10 g m⁻³, a reasonable value range for treated wastewater entering constructed wetlands based on the NAWDB (1993), while fixing the hydraulic loading rate at 5 cm day⁻¹ to investigate the change in phosphorus retention. As hydraulic loading rates increased from 2.5 to 15 cm day - 1 percent P removal (on average as mass) decreased by 29% when the inflow TP concentration was consistently kept at 2 g m^{-3} as shown in Table 5. The model estimated more than 60% phosphorus removal consistently when inflow TP concentration varied from 0.1 to 10 g m $^{-3}$ at 5 cm day $^{-1}$

Table 5 Simulations conducted over a 2-year period under various combinations of hydraulic and phosphorus-loading rates to design the pilot-scale wetland ($\approx 3 \text{ m} \times 7.8 \text{ m} \times 1.5 \text{ m}$) lined with FGD by-product in this study

	Inflow P concentration (g m $^{-3}$) P loading P removal (mass) (g m $^{-2}$ day $^{-1}$) (%)	2 0.05 74 0.1 0.005 63 2 0.1 63 3 0.15 63	2 0.2 31 2 0.3 45
	Water depth ^a Retention time ^a (m)	2.5(5) 2.5 2.5 2.5 2.5	2.5(0.83)
oduct in this study	Water depth ^a (m)	0.06(0.12) 0.12 0.12 0.12 0.12	0.25(0.12) $0.37(0.12)$
n×1.3 m) ined with FGD by-produc	Hydraulic loading $(cm day^{-1})$	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	10 15
mi (m č.1×m	Simulation	- 0 × 4 × v	0 /-

^a Values in parentheses show corresponding change between water depth and retention time.

of hydraulic loading rate (Table 5). Most simulations predicted more than 50% TP retention except the case where the hydraulic loading rate was 15 cm day⁻¹ and inflow TP concentration was 2 g m⁻³. Mean TP retention over a 2-year run of the model in this case was less than 50%, thus indicating hydraulic loading rate is the major determinant of phosphorus retention performance predicted by the model.

Retention time in Table 5 was calculated by a simple theoretical equation below (Mitsch and Gosselink, 2000).

$$t = V_{\rm p}/Q \tag{4}$$

where t is the theoretical retention time (day), V is the volume of water for surface flow wetland (m³), P is the porosity of medium, and ~ 1.0 for surface flow wetlands, Q is the flow rate through wetland (m³ day⁻¹) = $(Q_i + Q_o)/2$, where Q_i is the inflow and Q_o is the outflow.

Manipulating water depth can change calculated retention time. The pilot-scale FGD-lined wetlands are designed to control water depth so that we can change the retention time of water being treated in the wetlands. The values in parentheses in Table 5 for water depth and retention time show those possible changes. Based on those simulations explored we suggested design parame-

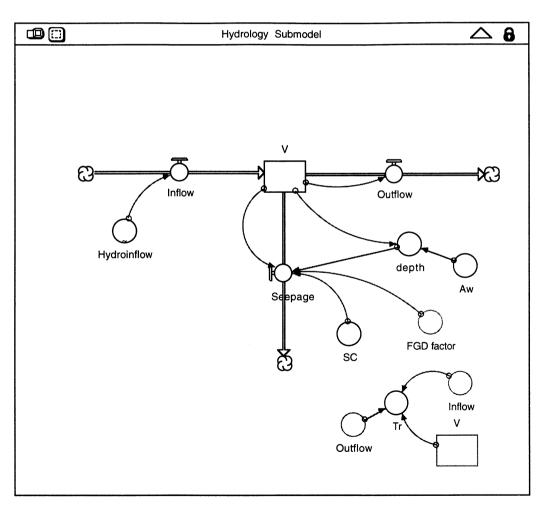


Fig. 6. STELLATM diagram of the ecologic-economic model.

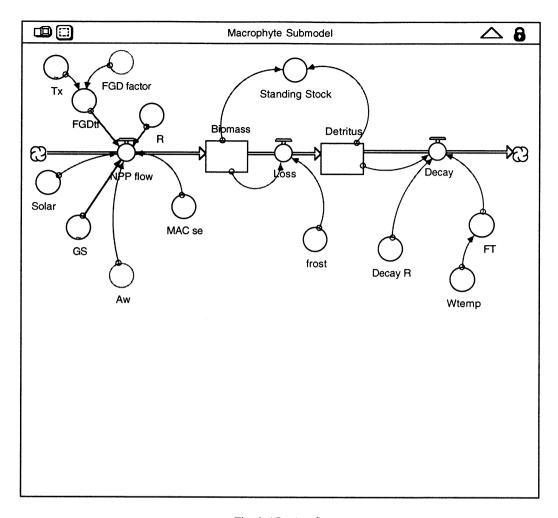


Fig. 6. (Continued)

ters for the pilot-scale experimental wetlands (Table 6). We chose 5 cm days⁻¹ as a conservative loading rate and 2–3 g m⁻³ typical of treated wastewater as inflow TP concentration. Water depth is designed up to 0.3 m in the pilot-scale wetland (Table 6), high enough to contain other aquatic life than just macrophytes, e.g. amphibians and benthic invertebrates.

5.4. Simulation of LCW with a liner

Table 7 shows the model simulations of LCWN with two different types of liners, either clay or FGD by-product. The same hydraulic loading

rate (9.8 cm day⁻¹) and inflow TP concentration (1.19 g m⁻³) as in the model validation were applied to those simulations. The differences from the validation simulation was no seepage effect in both simulations of LCWN with an either clay or a FGD liner. Moreover, additional P retention and early phytotoxicity from FGD by-product were applied to the model simulation with a FGD liner. A slight increase of water depth was observed in the simulations with a liner due to no seepage (Table 7). The no seepage effect included in the model simulation with an either clay or FGD liner resulted in more phosphorus in the water column of the LCW, thus potentially de-

creasing phosphorus retention since no more phosphorus could be removed through seepage (see Table 1). Based on this model structure, the LCW with a clay liner showed a slight decrease in its phosphorus retention relative to that with no liner (Table 7). More investigation is needed on the physicochemical properties of clay liner material being used for treatment wetlands. The pilot-

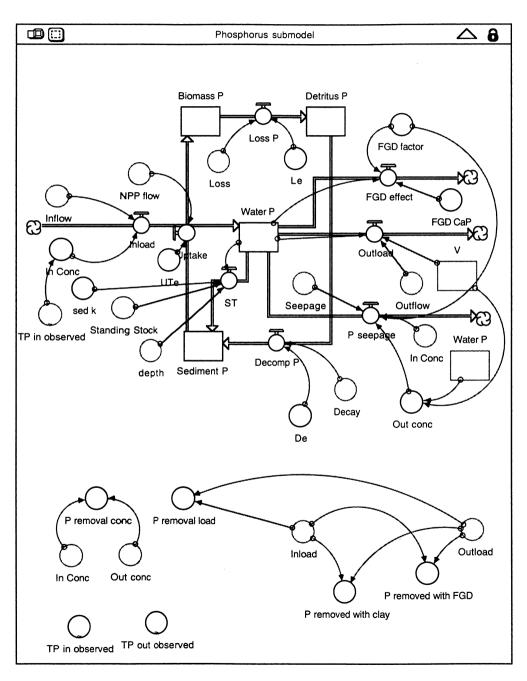


Fig. 6. (Continued)

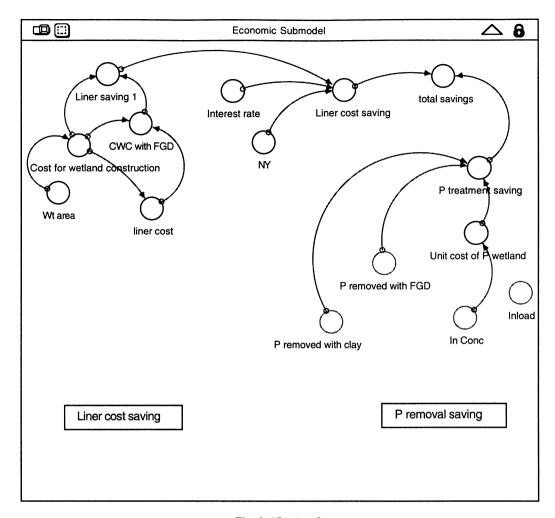


Fig. 6. (Continued)

scale wetlands lined with clay as a control to the FGD-lined ones may provide further information to be incorporated into the model as the experiment proceeds.

In the simulation of LCW with a FGD liner some changes of P dynamics were found compared to the simulation conducted without a FGD liner. Most importantly, biomass production was about 9% lower on average due to potential phytotoxicity applied in the model. This trend was much pronounced in the first year, showing about 14% lower biomass production as toxicity was modeled to mitigate over time and become negligible toward the end of the first year.

Phosphorus in the biomass (Biomass P) was also 9% lower than that in the simulation with no liner. Reduction in biomass production naturally induced a decrease in detritus by an average of 12%. As a result, standing stock, biomass plus detritus, showed a 10% decrease on average over a 2-year period, which may have influenced phosphorus retention performance of the wetland because the standing stock was modeled to influence phosphorus sedimentation. Higher percent TP removals both by concentration and by mass, however, were observed in the LCW simulated with a FGD liner compared to other simulations (Table 7). Amount of phosphorus in water column (Wa-

Table 6 Design parameters suggested for the pilot-scale FGD wetlands ($\approx 3~\text{m} \times 7.8~\text{m} \times 1.5~\text{m}$) at the Olentangy River Wetland Research Park (ORWRP)

Parameters	Suggested design		
Type of flow	Surface flow		
Hydrology			
Loading	5 cm days ⁻¹ (for>50% TP removal)		
Retention time	>1 day		
Water depth	0.1-0.3 m		
Phosphorus	$2-3 \text{ g m}^{-3}$		
loading			
Basin characteristics			
Cells (number)	Multiple (4)		
Planting material	Scirpus sp.		
Substrate material	On-site soil over FGD by-product		

ter P) showed a 10% additional decrease as a result of potentially enhanced Ca-P precipitation included in the simulation. Therefore, model predictions show that increased phosphorus retention efficiency by FGD by-products offset the initial phytotoxicity that could otherwise negatively influence phosphorus retention.

5.5. Economic estimations

Recycling FGD by-products in wetland treatment systems offers two possible cost savings; savings from both liner cost and phosphorus removal cost. Liner cost saving was estimated in the economic accounting submodel. This estimation presents only the cost of material for a liner, excluding cost for excavation and compaction procedures needed to install the liner. Based on the 30-year, 8% interest assumption, calculation with the model resulted in the liner cost saving of

Table 8
Potential liner cost saving by recycling FGD by-product relative to clay as a liner in the Licking County Wetland (LCW) under various combinations of interest rate and life expectancy

Life of wetland (years)	Interest rate (%)	Liner cost saving (US \$ ha ⁻¹ year ⁻¹)
30	8	1351
30	6	1105
30	10	1613
20	8	1549
40	8	1275

about US \$ 1400 ha⁻¹ years⁻¹ for the LCW (Table 8). A model equation calculating the cost of wetland construction is a function of wetland size, therefore the liner cost saving (20% of wetland construction cost assumed) will decrease proportionately to decreasing construction cost as the wetland being built gets bigger, following the so-called 'economy of scale'. Liner costs in general are variable, based on the quantity, thickness and type of material specified (Kadlec et al., 2000). The scenario tested above was based on the assumption that we would not find on-site soils with high-clay content suitable for use as a liner. However, our approach remains reasonable and practical even at sites that do have high-clay soils. Kadlec and Knight (1996) report that cost of testing and compaction, even with good soils in place, can exceed the costs of a 0.08-cm polyvinyl chloride (PVC) liner.

Sensitivity of liner cost saving to the choice of interest rate and life expectancy of the wetland system was also explored (Table 8). If a 6% interest rate were chosen instead of 8%, then the annualized cost saving of the liner would be 18.2% less or about US \$ 1100 ha⁻¹ year⁻¹. If a

Table 7
Mean water depth and phosphorus retention of the Licking County Wetland (LCW) with two different types of potential liner, clay and FGD by-product, over a 2-year period of model simulation

Simulation	Water depth (m)	P removal (concentration) (%)	P removal (mass) (%)
No liner ^a	0.25	24.8	34.7
Clay liner	0.27	23.2	21.9
FGD liner	0.27	32.9	37.3

^a The case of validation.

10% interest rate were applied, the cost saving would be 16.3% more or about US \$ 1600 ha⁻¹ year⁻¹. At a fixed 8% rate, decreasing the life expectancy of the wetland from 30 years to 20 years resulted in about 13% increase in the annualized cost saving, while increasing it to 40 years decreased the cost saving by about 6%. Therefore, it seems that the liner cost saving is sensitive to assumptions regarding interest rate and life expectancy of wetlands.

Fig. 5 combines the other potential cost saving, P treatment saving, with both percent phosphorus removal predicted by the model and unit cost of P removal along a gradient of inflow TP concentrations. P treatment saving relates closely with inflow TP concentration $(r^2 = 0.9944)$ and increases as the TP concentration of surface inflow increases (Fig. 5). P treatment saving can potentially get bigger than the liner cost saving with increasing TP inflow concentration, covering more than half of the total potential cost saving (Table 9). Two vertically dotted lines in Fig. 5 show the minimum and maximum values of inflow TP concentration observed for treatment wetlands in North America (NAWDB, 1993). showing how much saving can be possible in the LCW when the inflow TP concentration changes within that range. Table 9 presents total potential cost saving estimated from the FGD-lined LCW (6.4 ha), which was about US \$ 23 000 per year (US \$ 3552 ha⁻¹ year⁻¹ \times 6.4 ha) by the model.

6. Conclusions

Recycling FGD by-products as liners in constructed wetlands may be environmentally benefi-

Table 9
Potential total cost saving of recycling FGD as a liner in the Licking County Wetland (LCW; 6.4 ha) predicted by the simulation model over a 2-year run in this study

Liner cost saving ^a (US \$ year ⁻¹)	P treatment saving (US \$ year ⁻¹)	Total cost saving (US \$ year ⁻¹)
8646	14 086	22 732

^a Based on the 30 year, 8% interest assumption.

cial and potentially economical. As long as we have to haul liner material to the site where the wetland treatment system is being constructed, FGD wastes provide an economic edge over clay or commercial liner materials. Enhanced phosphorus retention consistently applied in the model simulations through this study, however, needs more verification since it was based on the shortterm, small-scale mesocosm studies. Therefore, recycling FGD wastes may or may not be as much economically feasible or cost-effective as projected in our study. A larger-scale, long-term wetland study with FGD wastes is currently underway to obtain more data, which may lead to better manifestation and applicability of our ecologic-economic model. Further studies of phosphorus dynamics in FGD-lined wetlands and full socioeconomic assessment of recycling FGD wastes in treatment wetlands are still needed. The unique ecologic-economic modeling approach taken in this study is nonetheless valid and provides a bridge between wetland ecology and economic aspects of recycling FGD wastes in constructed wetlands.

Acknowledgements

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Appendix A. STELLATM equations of the ecologic-economic model (Fig. 6)

```
Hydrology Submodel
V(t) = V(t-dt) + (Inflow-Outflow-
Seepage) *dt
INIT V=8000
```

```
INFLOWS:
Inflow=Hydroinflow
OUTFLOWS:
Outflow=(2.1e-4*V^2)+(0.6*V)
Seepage=IF(FGD_factor=1) THEN(0)
ELSE(V*SC/depth)
```

```
Aw = 32000
                                                                                  R*Aw
depth=V/Aw
                                                                                   OUTFLOWS:
                                                                                  Loss=Biomass*(0.0007+frost)
SC = 0.1
                                                                                  Detritus(t) = Detritus(t-dt) + (Loss-
Tr = V/((Inflow + Outflow)/2)*7
                                                                                  Decay)*dt
Hydroinflow = GRAPH (TIME) (1.00,
                                                            21969).
                                                                                   INIT Detritus = 1100000
(2.00, 21969), (3.00, 17010), (4.00, 21969), (5.00, 21969), (5.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.00, 21969), (6.0
21969), (6.00, 17143), (7.00, 14651), (8.00,
                                                                                   INFLOWS:
21969), (9.00, 19901), (10.0, 20881), (11.0,
                                                                                  Loss = Biomass*(0.0007 + frost)
27293), (12.0, 22442), (13.0, 12642), (14.0,
                                                                                   OUTFLOWS:
12747), (15.0, 23555), (16.0, 43190), (17.0,
                                                                                  Decay=Detritus*Decay_R*FT
7763), (18.0, 40012), (19.0, 26390), (20.0,
                                                                                  Decay_R = 0.035
13223), (21.0, 12054), (22.0, 18151), (23.0,
                                                                                   FGDtf = IF(FGD_factor = 1)
                                                                                                                                         THEN(Tx)
21175), (24.0, 17115), (25.0, 15022), (26.0,
                                                                                   ELSE(1)
15981), (27.0, 13328), (28.0, 19425), (29.0,
                                                                                   frost = PULSE(1.5, 41, 52)
24458), (30.0, 23849), (31.0, 20216), (32.0,
                                                                                   FT = 1.06^{(Wtemp-20)}
15022), (33.0, 13566), (34.0, 16058), (35.0,
                                                                                  MAC_se = 0.025
15498). (36.0. 14735). (37.0. 17171). (38.0.
                                                                                  R = 4.1
13699), (39.0, 11445), (40.0, 16933), (41.0,
                                                                                   Solar = 4000 - 2000 * COS(2*PI*(TIME)/
13146), (42.0, 14469), (43.0, 15953), (44.0,
                                                                                   52)
14812), (45.0, 15582), (46.0, 16401), (47.0,
                                                                                   Standing_Stock=Biomass+Detritus
21969), (48.0, 20006), (49.0, 22869), (50.0,
                                                                                   Wtemp = 15-13*COS(2*PI*(TIME)/52)
27741), (51.0, 21969), (52.0, 21969), (53.0,
                                                                                  GS = GRAPH (TIME)
20405), (54.0, 19768), (55.0, 27664), (56.0,
                                                                                  (1.00, 0.00), (2.00, 0.00), (3.00, 0.00), (4.00, 0.00)
26607), (57.0, 39004), (58.0, 24297), (59.0,
                                                                                  0.00), (5.00, 0.00), (6.00, 0.00), (7.00, 0.00),
25970), (60.0, 21462), (61.0, 36967), (62.0,
                                                                                   (8.00, 0.00), (9.00, 0.00), (10.0, 0.00), (11.0,
                                                                                  0.00), (12.0, 0.00), (13.0, 1.00), (14.0, 1.00),
28063), (63.0, 30814), (64.0, 28567), (65.0,
                                                                                   (15.0, 1.00), (16.0, 1.00), (17.0, 1.00), (18.0, 1.00)
23821), (66.0, 22470), (67.0, 24010), (68.0,
                                                                                   1.00), (19.0, 1.00), (20.0, 1.00), (21.0, 1.00),
21861), (69.0, 24353), (70.0, 25011), (71.0,
                                                                                   (22.0, 1.00), (23.0, 1.00), (24.0, 1.00), (25.0, 1.00)
27216), (72.0, 23926), (73.0, 23184), (74.0,
                                                                                   1.00), (26.0, 1.00), (27.0, 1.00), (28.0, 1.00),
33810), (75.0, 20167), (76.0, 67942), (77.0,
                                                                                   (29.0, 1.00), (30.0, 1.00), (31.0, 1.00), (32.0, 1.00)
20272), (78.0, 19971), (79.0, 19971), (80.0,
                                                                                   1.00), (33.0, 1.00), (34.0, 1.00), (35.0, 1.00),
19971), (81.0, 9151), (82.0, 18969), (83.0,
                                                                                   (36.0, 1.00), (37.0, 1.00), (38.0, 1.00), (39.0, 1.00)
10166), (84.0, 6466), (85.0, 8922), (86.0, 19971),
                                                                                  0.00), (40.0, 0.00), (41.0, 0.00), (42.0, 0.00),
(87.0, 23957), (88.0, 22183), (89.0, 20655), (90.0,
                                                                                   (43.0, 0.00), (44.0, 0.00), (45.0, 0.00), (46.0, 0.00)
23640), (91.0, 23957), (92.0, 27749), (93.0,
                                                                                  0.00), (47.0, 0.00), (48.0, 0.00), (49.0, 0.00),
22389), (94.0, 25021), (95.0, 19856), (96.0,
                                                                                   (50.0, 0.00), (51.0, 0.00), (52.0, 0.00), (53.0,
20354), (97.0, 31724), (98.0, 28642), (99.0,
                                                                                  0.00), (54.0, 0.00), (55.0, 0.00), (56.0, 0.00),
31724), (100, 35630), (101, 25021), (102, 19971),
                                                                                   (57.0, 0.00), (58.0, 0.00), (59.0, 0.00), (60.0, 0.00)
(103, 37700), (104, 32891)
                                                                                  0.00), (61.0, 0.00), (62.0, 0.00), (63.0, 0.00),
                                                                                   (64.0, 0.00), (65.0, 1.00), (66.0, 1.00), (67.0,
Macrophyte Submodel
                                                                                   1.00), (68.0, 1.00), (69.0, 1.00), (70.0, 1.00),
Biomass(t) = Biomass(t-dt) + (NPP_
                                                                                   (71.0, 1.00), (72.0, 1.00), (73.0, 1.00), (74.0,
flow-Loss)*dt
                                                                                   1.00), (75.0, 1.00), (76.0, 1.00), (77.0, 1.00),
INIT Biomass = 950000
                                                                                   (78.0, 1.00), (79.0, 1.00), (80.0, 1.00), (81.0,
                                                                                   1.00), (82.0, 1.00), (83.0, 1.00), (84.0, 1.00),
INFLOWS:
                                                                                  (85.0, 1.00), (86.0, 1.00), (87.0, 1.00), (88.0, 1.00)
NPP_flow=Solar*MAC_se*GS*FGDtf/
                                                                                  1.00), (89.0, 1.00), (90.0, 1.00), (91.0, 0.00),
```

(92.0, 0.00), (93.0, 0.00), (94.0, 0.00), (95.0, 0.00)0.00), (96.0, 0.00), (97.0, 0.00), (98.0, 0.00), (99.0, 0.00), (100, 0.00), (101, 0.00), (102, 0.00),(103, 0.00), (104, 0.00)Tx = GRAPH (TIME) (1.00, 0.8), (2.00, 0.8), (3.00, 0.8), (4.00, 0.8),(5.00, 0.8), (6.00, 0.8), (7.00, 0.8), (8.00, 0.8),(9.00, 0.8), (10.0, 0.8), (11.0, 0.8), (12.0, 0.8),(13.0, 0.8), (14.0, 0.8), (15.0, 0.8), (16.0, 0.8),(17.0, 0.8), (18.0, 0.8), (19.0, 0.8), (20.0, 0.8),(21.0, 0.8), (22.0, 0.808), (23.0, 0.816), (24.0, 0.816)0.824), (25.0, 0.832), (26.0, 0.84), (27.0, 0.848), (28.0, 0.852), (29.0, 0.86), (30.0, 0.868), (31.0,0.876), (32.0, 0.884), (33.0, 0.892), (34.0, 0.9), (35.0, 0.908), (36.0, 0.916), (37.0, 0.924), (38.0,0.932), (39.0, 0.94), (40.0, 0.948), (41.0, 0.952), (42.0, 0.96), (43.0, 0.968), (44.0, 0.976), (45.0, 0.968), (45.00.984), (46.0, 0.992), (47.0, 1.00), (48.0, 1.00), (49.0, 1.00), (50.0, 1.00), (51.0, 1.00), (52.0, 1.00)1.00), (53.0, 1.00), (54.0, 1.00), (55.0, 1.00), (56.0, 1.00), (57.0, 1.00), (58.0, 1.00), (59.0,1.00), (60.0, 1.00), (61.0, 1.00), (62.0, 1.00), (63.0, 1.00), (64.0, 1.00), (65.0, 1.00), (66.0, 1.00)1.00), (67.0, 1.00), (68.0, 1.00), (69.0, 1.00), (70.0, 1.00), (71.0, 1.00), (72.0, 1.00), (73.0,1.00), (74.0, 1.00), (75.0, 1.00), (76.0, 1.00), (77.0, 1.00), (78.0, 1.00), (79.0, 1.00), (80.0,1.00), (81.0, 1.00), (82.0, 1.00), (83.0, 1.00), (84.0, 1.00), (85.0, 1.00), (86.0, 1.00), (87.0, 1.00), (88.0, 1.00), (89.0, 1.00), (90.0, 1.00), (91.0, 1.00), (92.0, 1.00), (93.0, 1.00), (94.0, 1.00), (95.0, 1.00), (96.0, 1.00), (97.0, 1.00), (98.0, 1.00), (99.0, 1.00), (100, 1.00), (101, 1.00),(102, 1.00), (103, 1.00), (104, 1.00)

Phosphorus Submodel

 $Biomass_P(t) = Biomass_P(t-dt)$ +(Uptake-Loss_P)*dt INIT Biomass_P=1430

INFLOWS:

Uptake=NPP_flow*UTe OUTFLOWS: Loss_P=Le*Loss Detritus_P(t) = Detritus_P(t-dt) +(Loss_P-Decomp_P)*dt INIT Detritus_P=9400

INFLOWS: Loss P=Le*Loss OUTFLOWS: Decomp_P=Decay*De $Sediment_P(t) = Sediment_P(t-dt)$ + (Decomp_P+ST-Uptake)*dt INIT Sediment_P=1920000

INFLOWS:

Decomp_P=Decay*De ST=IF(Standing_Stock < 4000000) THEN(Water_P*sed_k/depth) ELSE((Standing_Stock*0.00000005 +sed_k)/depth*Water_P) OUTFLOWS: Uptake=NPP_flow*UTe Water_P(t) = Water_P(t-dt) +(Inload-Outload-ST-FGD_effect-P_seepage)*dt INIT Water_P=9600

INFLOWS:

Outload) *1.1

 $sed_k=0.1$

Inload = Inflow*In_Conc OUTFLOWS: Outload = Water _ P * Outflow / V ST=IF(Standing_Stock < 4000000) THEN(Water_P*sed_k/depth) ELSE((Standing_Stock*0.00000005 +sed_k)/depth*Water_P) FGD_effect = IF(FGD_factor = 1) (Water_P*FGD_CaP) ELSE(0) P_seepage = IF(FGD_factor = 1) THEN(0) ELSE(Seepage*(In_Conc+ Out_conc)/2) De = 0.0038 $FGD_CaP = 0.82$ $FGD_factor = 2$ In_Conc=TP_in_observed Le = 0.05Out_conc=Water_P/V P_removal_conc=(In_Conc-Out_ conc)/In_Conc*100 P_removal_load = (Inload-Outload)/ Inload*100 P_removed_with_clay=(Inload-Outload) P_removed_with_FGD = (InloadUTe = 0.0028TP_in_observed=GRAPH(TIME) (1.00, 1.19), (2.00, 1.19), (3.00, 2.15), (4.00,1.19), (5.00, 1.19), (6.00, 1.75), (7.00, 1.63), (8.00, 1.19), (9.00, 0.8), (10.0, 0.62), (11.0, 2.19),(12.0, 0.34), (13.0, 11.9), (14.0, 0.36), (15.0, 11.9)2.72), (16.0, 0.46), (17.0, 0.12), (18.0, 0.2), (19.0, 0.18), (20.0, 0.44), (21.0, 1.19), (22.0, 1.19), (23.0, 1.19), (24.0, 1.19), (25.0, 1.19), (26.0, 1.19)1.19), (27.0, 1.19), (28.0, 1.19), (29.0, 1.38), (30.0, 0.947), (31.0, 1.02), (32.0, 1.62), (33.0, 1.02)1.58), (34.0, 1.08), (35.0, 0.916), (36.0, 0.541), (37.0, 1.42), (38.0, 1.56), (39.0, 1.97), (40.0, 1.97)1.82), (41.0, 2.32), (42.0, 1.98), (43.0, 1.30), (44.0, 2.30), (45.0, 1.60), (46.0, 2.90), (47.0,1.19), (48.0, 0.88), (49.0, 1.50), (50.0, 1.00), (51.0, 1.19), (52.0, 1.19), (53.0, 1.30), (54.0, 1.19),2.00), (55.0, 2.09), (56.0, 2.09), (57.0, 2.43), (58.0, 0.68), (59.0, 1.60), (60.0, 2.09), (61.0, 0.2), (62.0, 0.247), (63.0, 0.79), (64.0, 1.73), (65.0, 0.79)0.43), (66.0, 0.57), (67.0, 0.56), (68.0, 1.71), (69.0, 0.79), (70.0, 0.2), (71.0, 0.3), (72.0, 0.98),(73.0, 0.75), (74.0, 1.19), (75.0, 0.849), (76.0, 0.849)1.75), (77.0, 0.265), (78.0, 0.328), (79.0, 0.371), (80.0, 0.343), (81.0, 0.877), (82.0, 0.139), (83.0,0.25), (84.0, 0.0714), (85.0, 0.156), (86.0, 0.275), (87.0, 0.391), (88.0, 2.02), (89.0, 1.11), (90.0, 1.39), (91.0, 1.58), (92.0, 0.728), (93.0, 0.447), (94.0, 1.32), (95.0, 0.462), (96.0, 0.368), (97.0,1.53), (98.0, 2.39), (99.0, 0.786), (100, 0.637), (101, 0.967), (102, 0.166), (103, 0.11), (104,0.447) TP_out_observed=GRAPH(TIME) (1.00, 0.952), (2.00, 0.952), (3.00, 0.68), (4.00,0.952), (5.00, 0.952), (6.00, 0.952), (7.00, 2.37), (8.00, 0.952), (9.00, 0.952), (10.0, 0.77), (11.0,1.02), (12.0, 0.84), (13.0, 1.04), (14.0, 0.29), (15.0, 0.11), (16.0, 0.51), (17.0, 0.952), (18.0, 0.952)0.74), (19.0, 0.37), (20.0, 0.19), (21.0, 0.4), (22.0, 0.74)1.84), (23.0, 1.43), (24.0, 0.952), (25.0, 0.952), (26.0, 0.952), (27.0, 0.952), (28.0, 0.952), (29.0,0.952), (30.0, 0.952), (31.0, 0.482), (32.0, 1.01), (33.0, 1.36), (34.0, 1.21), (35.0, 1.92), (36.0, 1.92)4.10), (37.0, 1.09), (38.0, 0.837), (39.0, 0.952), (40.0, 0.952), (41.0, 0.952), (42.0, 0.952), (43.0,0.952), (44.0, 0.952), (45.0, 1.10), (46.0, 1.30), (47.0, 0.952), (48.0, 0.952), (49.0, 0.55), (50.0,1.20), (51.0, 0.952), (52.0, 0.952), (53.0, 1.20),

(54.0, 2.20), (55.0, 1.76), (56.0, 1.08), (57.0, 1.83), (58.0, 0.75), (59.0, 1.31), (60.0, 1.70), (61.0, 0.93), (62.0, 0.673), (63.0, 4.87), (64.0, 0.82), (65.0, 0.85), (66.0, 0.49), (67.0, 0.93), (68.0, 1.32), (69.0, 1.13), (70.0, 0.83), (71.0, 0.23), (72.0, 1.09), (73.0, 0.9), (74.0, 0.37), (75.0, 0.365), (76.0, 2.56), (77.0, 0.51), (78.0, 0.489), (79.0, 0.53), (80.0, 0.39), (81.0, 0.406), (82.0, 0.883), (83.0, 0.201), (84.0, 0.724), (85.0, 0.603), (86.0, 0.214), (87.0, 0.405), (88.0, 0.374), (89.0, 0.37), (90.0, 0.532), (91.0, 0.634), (92.0, 1.24), (93.0, 0.972), (94.0, 0.395), (95.0, 0.621), (96.0, 0.605), (97.0, 0.551), (98.0, 1.12), (99.0, 0.789), (100, 0.675), (101, 0.201), (102, 1.15), (103, 0.253), (104, 0.425)

Economic Accounting Submodel Cost_for_wetland_construction $=196336*(Wt_area)^(-0.511)*Wt$ -area CWC_with_FGD=Cost_for_wetland -construction-liner_cost Interest_rate=0.08 liner_cost=Cost_for_wetland -construction*0.2 Liner_cost_saving=-PMT(Interest_ rate, NY, Liner_saving_1,0) Liner_saving_1=Cost_for_wetland -construction-CWC_with_FGD NY = 30P_treatment_saving = (P_removed_ with_FGD-P_removed_with_clay)* Unit_cost_of_P_wetland*52*2 total_savings=Liner_cost_saving +P_treatment_saving Unit_cost_of_P_wetland=0.1781 *In_Conc^(-0.7151) Wt_area=6.4

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